

5. SERVICE LINE MGRS.

By Date

Revisions

VA FORM 08-6231, JUN 1992 1

10. COTR

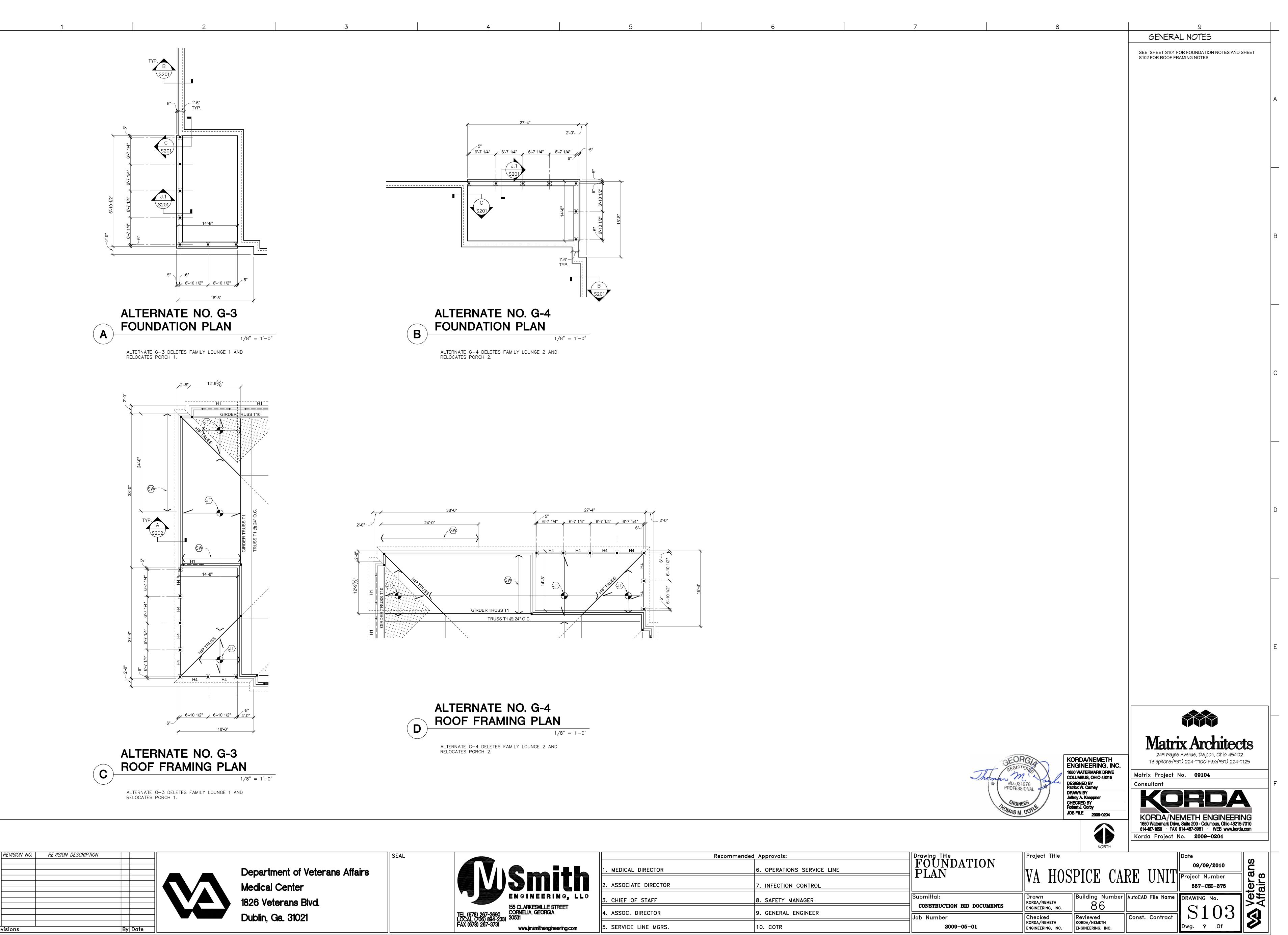
KORDA/NEMETH

ENGINEERING, INC.

2009-05-01

KORDA/NEMETH

ENGINEERING, INC.



5. SERVICE LINE MGRS.

10. COTR

Const. Contract

Checked KORDA/NEMETH

ENGINEERING, INC.

Job Number

2009-05-01

Reviewed KORDA/NEMETH

ENGINEERING, INC.

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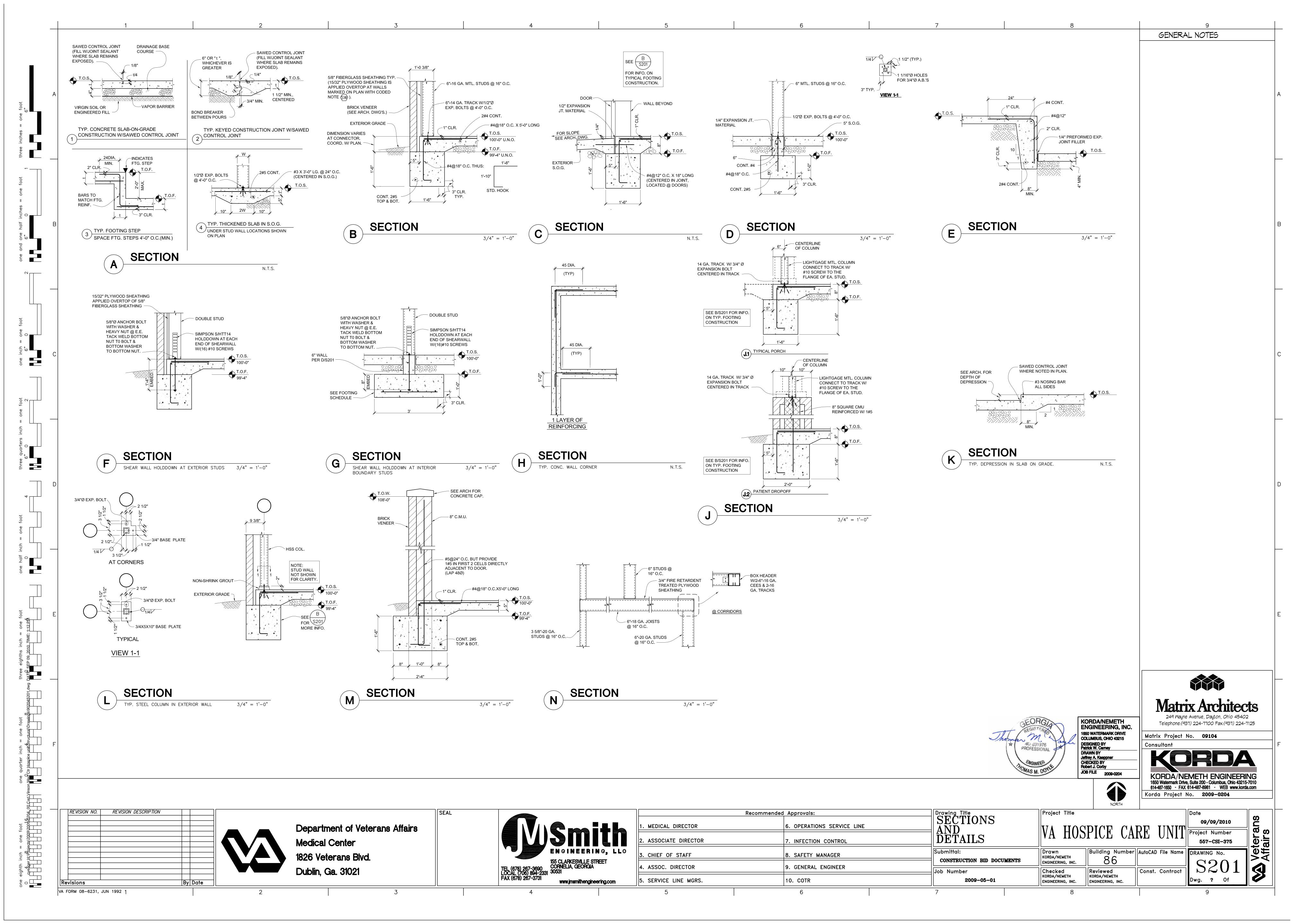
Revisions

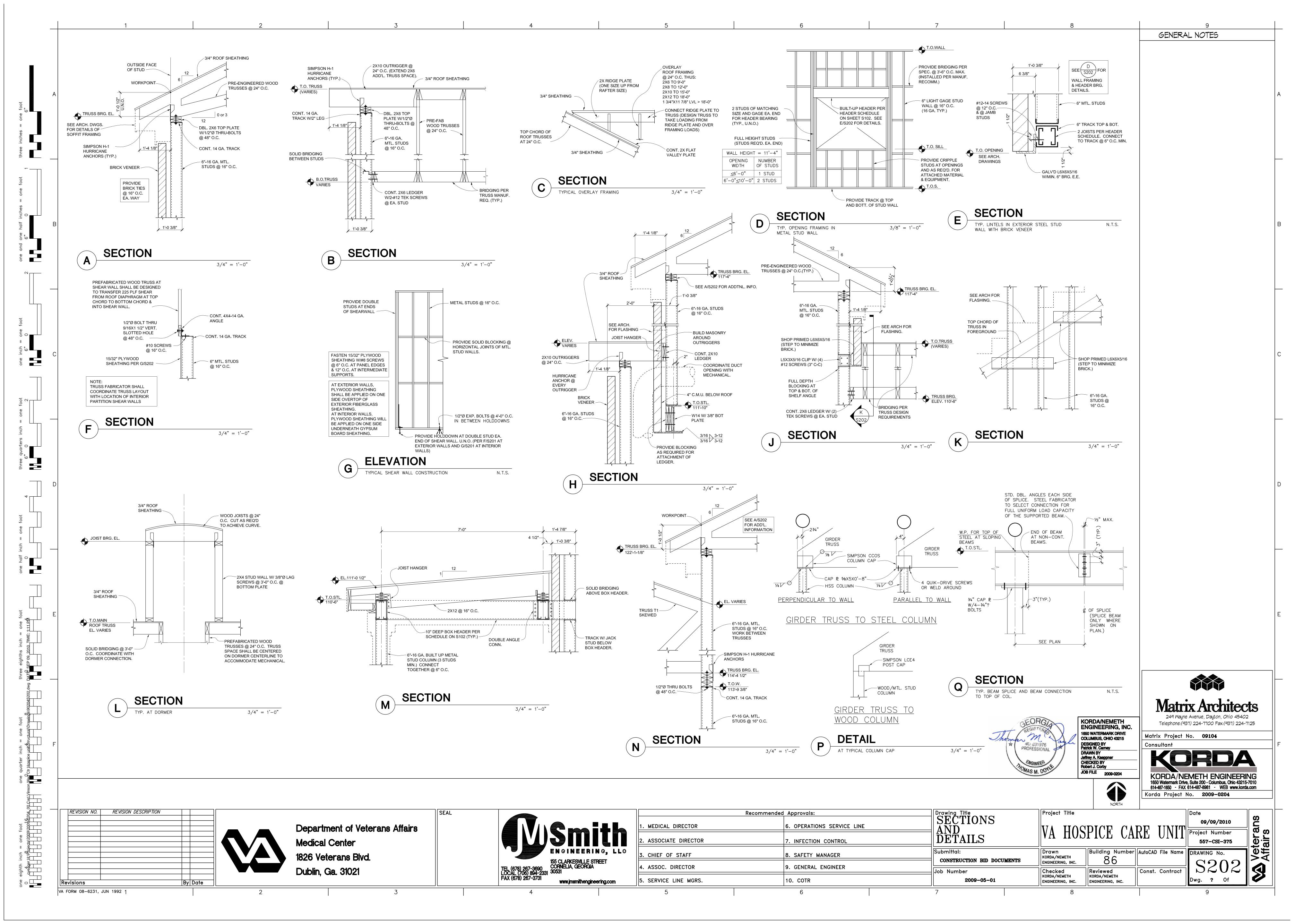
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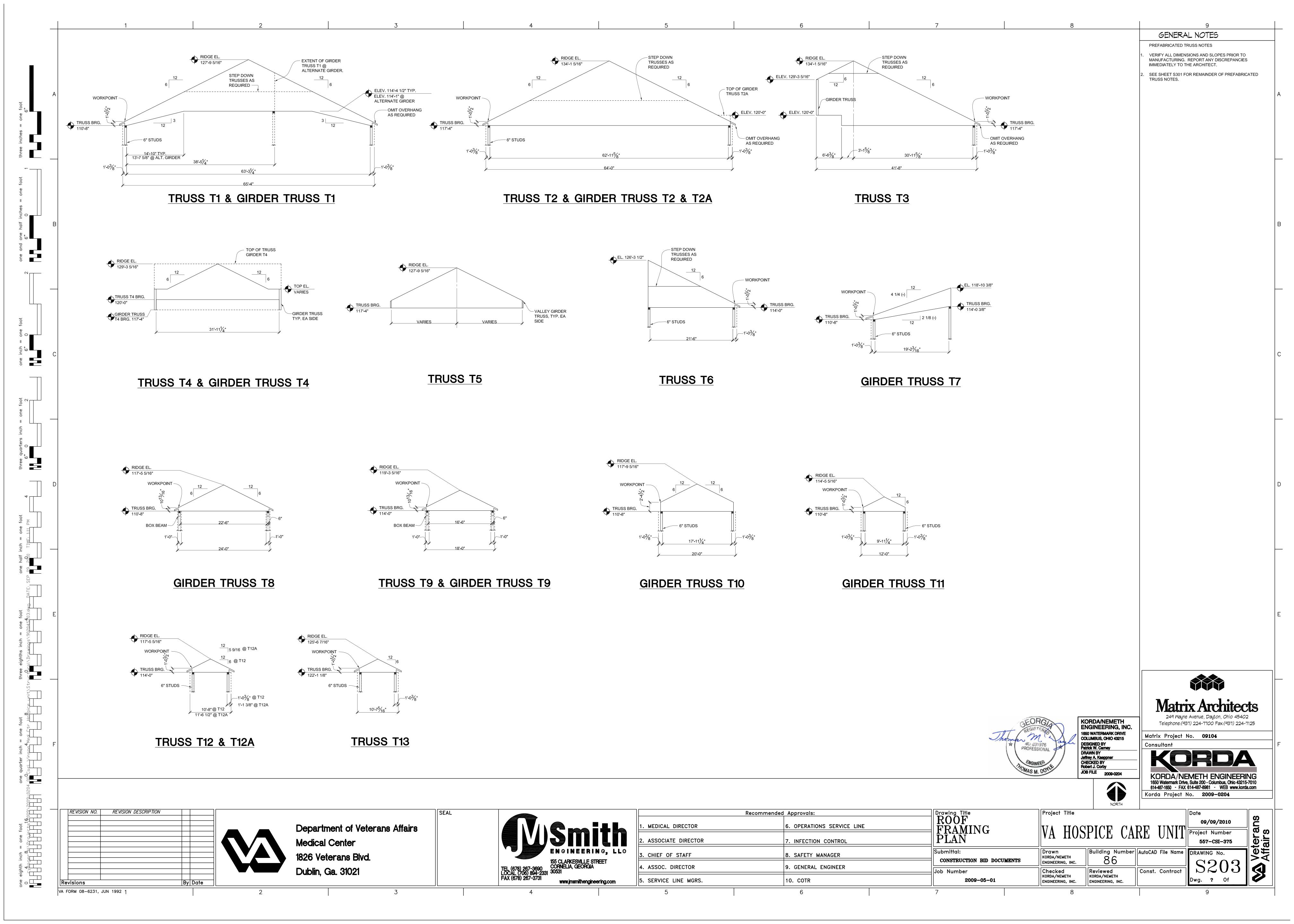
Dublin, Ga. 31021

By Date

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## STRUCTURAL NOTES

A. GENERAL

1. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE, AND TO ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS, AND THE ADEQUACY OF TEMPORARY OR INCOMPLETE CONNECTIONS, DURING ERECTION. THIS INCLUDES THE 8. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE ADDITION OF ANY SHORING, SHEETING, TEMPORARY GUYS, BRACING OR TIEDOWNS THAT MIGHT BE NECESSARY. SUCH MATERIAL IS NOT SHOWN ON THE DRAWINGS. IF APPLIED, THEY SHALL BE REMOVED AS CONDITIONS PERMIT, AND SHALL REMAIN THE CONTRACTOR'S PROPERTY. THE ENGINEER HAS NO EXPERTISE IN, AND TAKES NO RESPONSIBILITY FOR, CONSTRUCTION MEANS AND METHODS OR JOB SITE SAFETY DURING CONSTRUCTION. PROCESSING AND/OR APPROVING SUBMITTALS MADE BY THE CONTRACTOR WHICH MAY CONTAIN INFORMATION RELATED TO CONSTRUCTION METHODS OR SAFETY ISSUES, OR PARTICIPATION IN MEETINGS WHERE SUCH ISSUES MIGHT BE DISCUSSED, SHALL NOT BE CONSTRUED AS VOLUNTARY ASSUMPTION BY THE ENGINEER OF ANY RESPONSIBILITY FOR SAFETY PROCEDURES.

IT IS SOLELY THE RESPONSIBILITY OF EACH CONTRACTOR TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE ENGINEER IS NOT ENGAGED IN, AND DOES NOT SUPERVISE,

3. EQUIPMENT FRAMING LOADS, OPENINGS AND STRUCTURE IN ANY WAY RELATED TO HVAC, PLUMBING, OR ELECTRICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL COORDINATE THIS INFORMATION WITH THE INVOLVED TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN THESE REQUIREMENTS TO BE BORNE BY THE APPROPRIATE CONTRACTOR.

4. SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE

GOVERNING CODES:

GEORGIA BUILDING CODE b. VA GUIDELINES - NURSING HOMES & HOSPITALS

STRICTEST PROVISION SHALL GOVERN.

6. EXISTING BUILDING: PROVIDE TEMPORARY SUPPORTS AND OTHER MEASURES AS REQUIRED TO PREVENT DAMAGE TO THE EXISTING BUILDING DURING CONSTRUCTION. FIELD VERIFY ALL EXISTING DIMENSIONS AND ELEVATIONS WHICH

7. DESIGN ROOF LIVE LOAD: 20 PSF a. GROUND SNOW LOAD (Pg) = 5 PSF

AFFECT THE NEW CONSTRUCTION.

b. FLAT ROOF SNOW LOAD (Pf) = 6 PSF c. SNOW EXPOSURE FACTOR (Ce) = 1.0

d. SNOW LOAD IMPORTANCE FACTOR (I) = 1.2 e. ROOF LIVE LOAD = 20 PSF

8. DESIGN FLOOR LIVE LOADS: a. SLABS ON GRADE = 250 PSF

WIND DESIGN PARAMETERS

a. BASIC WIND SPEED = 95 MPH b. WIND LOAD IMPORTANCE FACTOR = 1.15

WIND EXPOSURE = EXPOSURE B INTERNAL PRESSURE COEFFICIENTS = +/-0.18 e. MAIN WIND DESIGN PRESSURE = 16 PSF

10. SEISMIC DESIGN PARAMETERS

a. IMPORTANCE FACTOR = 1.5 b. MAPPED SPECTRAL RESPONSE ACCELERATION: Ss=0.22, S1=0.08

d. SPECTRAL RESPONSE COEFFICIENTS: SDS = 0.235, SD1 = 0.128

SEISMIC DESIGN CATEGORY = CATEGORY ( f. BASIC SEISMIC FORCE RESISTING SYSTEM - LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS.

SEISMIC RESPONSE COEFFICIENT (Cs) = 0.05 h. RESPONSE MODIFICATION FACTOR (R) =  $6\frac{1}{2}$ DEFLECTION AMPLIFICATION FACTOR (Cd) = 4

DESIGN BASE SHEAR = 29 KIPS k. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

B. REINFORCED CONCRETE

a. SPECIFICATIONS: IN GENERAL, COMPLY WITH ACI 301-05 "SPECIFICATIONS FOR STRUCTURAL CONCRETE." b. STRUCTURAL CONCRETE

3500

CLASS LOCATION FOOTINGS

> AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED. EXTERIOR SLABS ON GRADE, (with air)

AND ALL EXTERIOR CONCRETE NOT OTHERWISE IDENTIFIED.

BACKFILL BELOW FOOTINGS

INTERIOR SLABS ON GRADE,

c. ALL DEFORMED REINFORCING BARS: FY = 60,000

FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.

a. INCLUDE AN ALLOWANCE IN THE BID TO PROVIDE AND INSTALL AN ADDITIONAL 1/2 TON OF REINFORCING BARS. MATERIAL TO BE USED AND ITS APPLICATION SHALL BE DETERMINED BY THE ARCHITECT. BENT BARS, IF REQUIRED, SHALL BE BENT IN THE SHOP, UNLESS OTHERWISE APPROVED.

PROVIDE SUPPORTS AS REQUIRED TO MAINTAIN ALIGNMENT OF SCHEDULED

REINFORCING. SUCH SUPPORTS ARE TO BE REFLECTED IN THE BID, AND ARE

a. OPENINGS SHOWN ARE FOR BIDDING PURPOSES ONLY. COORDINATE THEIR

NOT PART OF THE CONTINGENCY QUANTITY LISTED ABOVE.

EXACT SIZES AND LOCATIONS WITH HVAC, PLUMBING, AND OTHER REQUIREMENTS BEFORE PROCEEDING WITH WORK. b. IF ANY OPENING NOT SHOWN ON THE PLANS IS REQUIRED, SECURE

APPROVAL OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING.

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a. DOWELS IN FOOTINGS TO MATCH VERTICAL WALL REINFORCING. UNLESS SHOWN OTHERWISE, PROVIDE THE FOLLOWING WALL REINFORCING: VERTICAL STEEL WALL THICKNESS HORIZONTAL STEEL 6 AND 8 INCHES #4 @ 12 INCHES #4 @ 18 INCHES 10 AND 12 INCHES #4 @ 16 INCHES EF #4 @ 18 INCHES EF

45 BAR DIAMETERS. d. CAST IN CONTINUOUS DOVETAIL ANCHOR SLOTS ON VERTICAL SURFACES WHERE MASONRY ABUTS, 16 INCHES O.C. FOR PARALLEL SURFACES, AT

PROVIDE CORNER BARS AT WALL CORNERS TO MATCH HORIZONTAL

REINFORCING. MINIMUM LAP LENGTH WITH HORIZONTAL REINFORCEMENT.

CENTERLINE OF MASONRY FOR PERPENDICULAR SURFACES. e. PROVIDE LEAN CONCRETE (CLASS IV) UNDER FOUNDATIONS FOR ACCIDENTAL OVER-EXCAVATION, SOFT SPOTS AND TRENCHES.

6. SPLICES: UNLESS NOTED OTHERWISE, MINIMUM LAP SPLICE LENGTHS TO BE AS

a. HORIZONTAL BARS IN SLABS & 35 DIAMETERS

FOOTINGS b. HORIZONTAL BARS IN WALLS 45 DIAMETERS 7. CONSTRUCTION JOINTS:

a. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR AS APPROVED BY THE STRUCTURAL ENGINEER. ALL CONSTRUCTION JOINTS ARE TO BE KEYED. KEYWAYS SHALL BE 1-1/2 INCHES DEEP X 1/3 MEMBER THICKNESS.

CONCRETE COVER AS FOLLOWS: a. CONCRETE CAST AGAINST AND PERMANENTLY

EXPOSED TO EARTH: 3 INCHES b. CONCRETE EXPOSED TO EARTH OR WEATHER: 1-1/2 INCHES #5 BARS AND SMALLER OTHERS 2 INCHES c. CONCRETE NOT EXPOSED TO EARTH OR

WEATHER: SLABS, WALLS #11 BARS AND SMALLER 1 INCH

9. MISCELLANEOUS:

a. GROUT UNDER COLUMN BASE PLATES SHALL BE NON-SHRINKING TYPE. THE USE OF LEVELING PLATES AT COLUMN BASES IS PROHIBITED. GROUT BELOW COLUMN BASE PLATES IS TO BE INSTALLED ONLY AFTER THE STEEL IS PLUMBED.

1. SPECIFICATIONS: MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-08," PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE, DETROIT, MICHIGAN, EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE CONTRACT DOCUMENTS.

a. CONCRETE BLOCK: ASTM C90. MINIMUM NET AREA COMPRESSIVE STRENGTH OF C.M.U. = 1900 PSI.

b. MORTAR: ASTM C270 (USING THE PROPERTY SPECIFICATION METHOD, PARAGRAPH 3.2), TYPE S. MINIMUM COMPRESSIVE STRENGTH = 1800 PSI. c. BOND BEAM AND CORE FILL: ASTM C476, COARSE OR FINE TYPE, PLACED PER ACI 530.1. TABLE 7.

d. JOINT REINFORCING: HOT-DIPPED GALVANIZED FINISH, 9 GAGE MINIMUM SIDE WIRES AND CROSS WIRES. e. BAR REINFORCING: ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.

f. WIRE TIES AND ANCHORS: RECTANGULAR TYPE, 3/16" DIAMETER WIRE TIES (HOT DIPPED GALVANIZED).

3. REINFORCED MASONRY: WHERE VERTICAL BARS ARE TO BE GROUTED INTO CORES, THE FOLLOWING REQUIREMENTS APPLY: a. PROVIDE DOWELS FROM FOOTING, SAME SIZE AND SPACING AS WALL BARS. LAP

12 INCHES MINIMUM WITH WALL BAR. EMBED INTO FOOTING 9 INCHES. b. PROVIDE A CONTINUOUS VERTICAL CAVITY, AT LEAST 3" X 4" IN SIZE, FREE OF MORTAR DROPPING. c. PROVIDE REBAR ALIGNMENT DEVICES AT A MAXIMUM SPACING OF 96 BAR

DIAMETERS (MINIMUM OF 2 PER BAR). d. AT SPLICES IN VERTICAL BARS, PROVIDE 48 DIAMETER LAP. e. ALL REINFORCEMENT MUST BE INSTALLED AND SECURELY ANCHORED IN PLACE PRIOR TO PLACEMENT OF GROUT.

f. MAXIMUM HEIGHT OF GROUT LIFT = 5'-0". g. ALL C.M.U.'S USED IN REINFORCED MASONRY SHALL BE TWO CELL UNITS.

4. MISCELLANEOUS: a. EXCEPT FOR INSULATED CAVITY WALLS, VERTICAL COLLAR JOINTS SHALL BE FILLED SOLID WITH MORTAR OR GROUT. b. PROVIDE MINIMUM TWO COURSES 100% SOLID BEARING UNDER BEARING

ELEMENTS, UNLESS DETAILED OTHERWISE. c. FILL CORE SOLID AROUND ANCHOR BOLTS. d. PROVIDE 100% SOLID BLOCKS OR SOLIDLY-FILLED HOLLOW BLOCKS FOR AT

LEAST 4" ALL AROUND ALL EXPANSION BOLTS. e. HOLLOW MASONRY UNITS TO BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAID WITH FULL HEAD AND BED JOINTS.

f. PROVIDE JOINT REINFORCING AT 16 INCHES. g. LAP JOINT REINFORCING 6 INCHES FOR STANDARD.

h. WHERE MASONRY UNITS ARE USED ABOVE HOLLOW UNITS OF A DIFFERENT THICKNESS, PROVIDE A CONTINUOUS COURSE OF 100% SOLID MASONRY AT LEAST 8 INCHES HIGH BELOW TRANSITION. i. IN ALL MULTIPLE WYTHE WALLS AND CAVITY WALLS PROVIDE RECTANGULAR 3/16" DIA. WIRE WALL TIES (FIXED OR ADJUSTABLE) AT 16" O.C. HORIZONTALLY AND

LINTEL NOTES

1. PROVIDE LINTELS OVER ALL OPENINGS IN MASONRY WALLS. REFER TO ARCHITECTURAL AND HVAC DRAWINGS FOR LOCATION, NUMBER AND SIZES OF OPENINGS. FOR LINTELS NOT LABELED OR SHOWN ON STRUCTURAL DRAWINGS, APPLY NOTES 2 THROUGH 6.

2. FOR LINTELS OVER OPENINGS 6'-0" WIDE OR LESS, PROVIDE THE FOLLOWING FOR EACH 4 INCHES OF WALL THICKNESS (USE 6 INCHES MINIMUM BEARING EACH END); IN CAVITY WALLS, ADD L5 X 5 X 5/16 FOR BRICK SUPPORT WITH 6" BEARING EACH END AND CONTINUOUS BOTTOM PLATE 5/16 X (WALL "T"-1/2") AND STOP PLATE 1/8" SHORT

MASONRY OPENINGS SECTION L 3-1/2 X 3-1/2 X 5/16 4'-1" TO 5'-6" L 4 X 3-1/2 X 5/16 LLV 5'-7" TO 6'-0" L 5 X 3-1/2 X 5/16 LLV

W 8 X 21

3. FOR LINTELS OVER OPENINGS GREATER THAN 6'-0" WIDE, PROVIDE THE FOLLOWING BEAM SECTIONS WITH 7-1/2 INCH MINIMUM BEARING EACH END; ADD CONTINUOUS BOTTOM PLATE 5/16 X (WALL "T"-1/2") AND STOP PLATE 1/8" SHORT OF JAMBS; IN CAVITY WALLS ADD CONT. L5 X 5 X 5/16 WITH 7-1/2 INCH MINIMUM BEARING EACH END:

MASONRY OPENING SECTION 6'-1" TO 6'-6" W 8 X 13 6'-7" TO 7'-11" W 8 X 18

4. CENTER WIDE FLANGE BEAM LINTELS ON FULL WIDTH OF BONDED MULTI-WYTHE WALLS. CENTER WIDE FLANGE BEAM LINTELS ON BLOCK BACK-UP WYTHE IN CAVITY WALLS, EXCEPT WHERE OTHERWISE SHOWN ON DRAWINGS.

E. STRUCTURAL STEEL

8'-0" TO 12'-0"

1. THE STEEL FRAME AS DESIGNED IS A NON-SELF-SUPPORTING STEEL FRAME. COORDINATE THE ERECTION WITH THE INSTALLATION OF OTHER BUILDING ELEMENTS REQUIRED FOR THE STRUCTURE'S STABILITY. THESE ELEMENTS INCLUDE SLABS, WOOD SHEATHING, WOOD TRUSSES, AND LIGHT GAGE METAL FRAMING.

a. STRUCTURAL STEEL CHANNEL, ANGLES, PLATES, ETC.: ASTM A36, FY = 36 KSI; STRUCTURAL STEEL WIDE FLANGES: ASTM A572 OR ASTM A992, FY = 50 KSI; HIGH STRENGTH BOLTS: ASTM A325 OR A490; ANCHOR BOLTS: ASTM A307 OR A36; ELECTRODES: SERIES E70; STRUCTURAL PIPES OR ROUND TUBING: ASTM A53 OR A501; FY = 35 KSI MIN; SQUARE AND RECTANGULAR TUBING: ASTM A500, FY = 46 KSI: EXPANSION BOLTS: HILTI "KWIK-BOLT TZ." SIMPSON STRONG-TIE "STRONG-BOLT" OR APPROVED EQUAL. ADHESIVE ANCHORS: HILTI "HIT-ICE/HIT HY 150," SIMPSON STRONG-TIE "ACRYLIC-TIE," ITW RED-HEAD "A7 ACRYLIC."

3. SPECIFICATION: WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AWS D1.1. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION TO BE GOVERNED BY:

a. AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS

b. AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (MARCH 18, 2005). c. STRUCTURAL WELDING CODE, AWS D1.1/D1.1 M: 2008 OF THE AMERICAN WELDING

d. SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS (JUNE 30, 2004).

4. CONNECTIONS:

a. FIELD CONNECTIONS TO BE BOLTED. SHOP CONNECTIONS TO BE WELDED OR BOLTED. CONNECTIONS TO BE SELECTED BY THE FABRICATOR TO DEVELOP THE FULL UNIFORM LOAD CAPACITY OF THE MEMBER OR FORCES SHOWN ON PLANS, WHICHEVER IS GREATER. UNLESS INDICATED OTHERWISE, ALL CONNECTIONS MAY BE DOUBLE ANGLE CONNECTIONS OR SINGLE PLATE SHEAR CONNECTIONS. FOLLOW INSTRUCTIONS ON DRAWINGS FOR GENERAL ARRANGEMENT OR PARTICULAR

b. ALL BOLTS IN KNEE BRACED FRAMES SHALL BE SLIP CRITICAL TYPE OR DESIGNED AS BEARING TYPE BOLTS IN STANDARD HOLES AND PRETENSIONED TO SLIP-CRITICAL CONDITION. USE STANDARD HOLES OR SHORT-SLOTTED HOLES TRANSVERSE TO THE DIRECTION OF THE LOAD AND SELECT THE NUMBER OF BOLTS FOR A SERVICEABILITY LIMIT STATE (IF OVER SIZED HOLES OR SLOTS PARALLEL TO THE LOAD ARE PERMITTED SELECT THE NUMBER OF BOLTS FOR A STRENGTH LIMIT

GALVANIZING: ALL SHELF ANGLES, LINTELS IN EXTERIOR WALLS, ALL EXTERIOR STEEL EXPOSED TO THE ELEMENTS, AND ALL ITEMS INDICATED ON THE DRAWINGS AS "GALVANIZED" SHALL BE GALVANIZED.

PAINT: a. DO NOT PAINT STEEL OR ANCHOR BOLTS WHICH WILL BE GALVANIZED, ENCASED IN CONCRETE, STEEL THAT WILL RECEIVE SPRAYED-ON FIREPROOFING OR ANY STEEL NOT EXPOSED TO VIEW IN THE FINISHED STRUCTURE.

MISCELLANEOUS: a. THE COLUMNS, ANCHOR BOLTS, BASE PLATES AND FOUNDATIONS ARE DESIGNED TO RESIST A GRAVITY LOAD MOMENT OF 675 FOOT-POUNDS DURING ERECTION, IF SHIM PACKS ARE PROVIDED AT THE EXTREME EDGES ALONG ALL FOUR SIDES OF THE BASE PLATE. FOR SAFETY CONSIDERATIONS DURING ERECTION, SEE

REINFORCED CONCRETE NOTES ABOVE FOR GROUT AND GROUTING

STRUCTURAL NOTE A1. b. PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS. OBTAIN PRIOR APPROVAL. THE USE OF LEVELING PLATES AT COLUMN BASES IS PROHIBITED. SEE THE

REQUIREMENTS. d. STEEL BELOW GRADE IS TO BE PROTECTED BY A MINIMUM OF 3 INCHES OF CONCRETE. e. PROVIDE WASHER AND HEAVY NUT AT ALL ANCHOR BOLTS (BOTH ENDS).

FINISH ENDS OF ALL COLUMNS, STIFFENERS AND ALL OTHER MEMBERS IN DIRECT

PROVIDE BOLT HOLES FOR TOP PLATES BOLTED TO BEAMS. EMBEDMENT LENGTH OF EXPANSION BOLTS INTO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS: 1/2 INCH DIAMETER BOLTS — 3-1/2 INCHES EMBEDMENT

8 CONTINGENCY:

 a. INCLUDE AN ALLOWANCE IN THE BID TO PROVIDE AND ERECT AN ADDITIONAL 1/2 TON OF STRUCTURAL AND/OR MISCELLANEOUS STEEL (SHAPES, ANGLES, PLATES, ETC.). MATERIAL TO BE USED AND ITS APPLICATION SHALL BE DETERMINED BY THE ARCHITECT. CONNECTIONS, IF REQUIRED, SHALL BE FIELD-WELDED.

G. LIGHT GAGE METAL FRAMING

STUDS WHICH SERVE AS

1. ALL STUDS USED FOR EXTERIOR WALL FRAMING SHALL BE 6" METAL "C" STUDS SPACED 16" O.C. AS INDICATED BELOW:

LOCATION STUDS WHICH SERVE AS BACKUP FOR FACE BRICK

3/4 INCH DIAMETER BOLTS — 5 INCHES EMBEDMENT

BACKUP FOR WOOD PANEL 20 GAGE 1.74 IN4 2. ALL STUD CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR THE REACTIONS INDUCED BY THE LOADS INDICATED BELOW.

3. HORIZONTAL WIND LOAD = 30 PSF (INWARD OR OUTWARD).

OPENINGS THROUGH STUD WALLS. a. OPENINGS 12'-0" OR LESS IN WIDTH SHALL BE FRAMED WITH LIGHT GAGE FRAMING MEMBERS AS SHOWN IN SECTIONS D AND E/S202. b. WALL SECTIONS OVER OPENINGS GREATER THAN 12'-0" IN WIDTH SHALL BE FRAMED AS INDICATED ON THE DRAWINGS.

5. BOTTOM OF STUD WALLS SHALL BE ANCHORED TO THE SLAB WITH 1/2" DIAMETER EXPANSION BOLTS AT A MAXIMUM SPACING OF 4'-0" ON CENTER.

H. STRUCTURAL LUMBER

a. STRUCTURAL LUMBER: SOUTHERN PINE #2, ALLOWABLE STRESSES PER THE

SEAL

NATIONAL DESIGN SPECIFICATION SUPPLEMENT 2005 EDITION; 19% MAX. M.C. b. PLYWOOD: FOR ROOFS AND WALLS: C-D PLUGGED, STRUCTURAL I, EXPOSURE 1 5 PLY, WITH PANEL INDEX OF 24/0; 15/32 INCH THICK FOR WALLS, 3/4 INCH THICK WITH PLYWOOD CLIPS FOR ROOF SHEATHING. FOR PLATFORMS: C-D PLUGGED, STURD-I-FLOOR, EXPOSURE 1, WITH PANEL INDEX OF 16 OC; 1/2 INCH THICK. c. OSB: FOR WALLS: 15/32 INCH THICK WITH PANEL INDEX W24, EXPOSURE 1. FOR ROOFS: 3/4 INCH THICK WITH PANEL INDEX 1R24, EXPOSURE 1. FOR FLOORS: 1/2

INCH THICK, STURD-I-FLOOR WITH PANEL INDEX OF 1F24, EXPOSURE 1.

SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST EDITION OF: a. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. b. U.S. PRODUCT STANDARD PS1

CONNECTIONS: a. JOISTS TO SIDES OF BEAMS: 16 GA. GALVANIZED STD. JOIST HANGERS, UNLESS

SHOWN OTHERWISE. b. JOISTS AND TRUSSES TO TOPS OF WALLS AND BEAMS: 18 GA. GALVANIZED HURRICANE ANCHORS. c. PLYWOOD OR OSB TO PLATFORM JOISTS: SCREWED - USE #8 SCREWS AT 6

INCHES O/C AT PANEL EDGES AND 12 INCHES O/C AT INTERMEDIATE SUPPORTS. d. PLYWOOD OR OSB TO ROOF TRUSSES OR RAFTERS: NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O/C AT PANEL EDGES AND 12 INCHES O/C AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MID-SPAN OF PLYWOOD BETWEEN SUPPORTS.

MISCELLANEOUS: a. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O/C MAX. FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JOIST AND RAFTER BEARING. b. PROVIDE AND INSTALL BRIDGING FOR PREFABRICATED WOOD TRUSSES AS

INDICATED ON THE TRUSS MANUFACTURER'S APPROVED SHOP DRAWINGS.

I. PREFABRICATED WOOD TRUSSES

a. LUMBER: SOUTHERN PINE #2, ALLOWABLE STRESSES PER THE NATIONAL DESIGN SPECIFICATION SUPPLEMENT, 2005 EDITION; 19% MAX. M.C. b. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL, ASTM A653, GRADE A, COATING CLASS G60 PER ASTM A653 TYPICALLY. USE COATINGS CLASS G185 WHEN FRT IS USED. MANUFACTURE WITH HOLES, PLUGS, TEETH OR PRONGS UNIFORMLY SPACED AND FORMED.

DESIGN: a. TOP CHORD LIVE LOAD: TOP CHORD DEAD LOAD: 10 PSF BOTTOM CHORD LIVE LOAD: 10 PSF BOTTOM CHORD DEAD LOAD (NON-CONCURRENT): 15 PSF

NET WIND UPLIFT: b. FINAL DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN GEORGIA, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.

c. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. d. MEMBER SIZES SHOWN ARE MINIMUM SIZES.

e. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360. f. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.

a. BOLT TOP CHORDS OF ALL MULTIPLE TRUSSES TOGETHER WITH 1/2" DIAMETER BOLTS AT 4'-0" O.C. BOLT WEB MEMBERS TOGETHER WITH 1/2" DIAMETER BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS

b. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL STAMPED TRUSS

(GBC SECTION 2303.4.1.2). ABBREVIATIONS: B. = BOTTOM; B.L. = BRICK LEDGE EL; C.I.P. = CAST IN PLACE; C.M.U. = CONCRETE MASONRY UNIT; (E) = EXISTING; E.E. = EACH END; E.F. = EACH FACE; E.W. = EACH WAY; GALV. = GALVANIZED; LLH = LONG LEG HORIZONTAL; LLV = LONG LEG VERTICAL; N.T.S. = NOT TO SCALE; O.C. = ON CENTER; S.O.G. = SLAB ON GRADE; T. =

TOP; T.O.F. = TOP OF FOOTING ELEVATION; T.O.P. = TOP OF PIER ELEVATION; T.O.S. =

TOP OF SLAB ELEVATION; T.O.STL. = TOP OF STEEL ELEVATION; T.O.W. = TOP OF

WALL ELEVATION; TYP. = TYPICAL; U.N.O. = UNLESS NOTED OTHERWISE; W.W.F. =

FABRICATION DRAWINGS AND A TRUSS LAYOUT DRAWING TO THE AUTHORITY

HAVING JURISDICTION. TRUSSES SHALL NOT BE INSTALLED PRIOR TO APPROVAL

K. LEGEND:

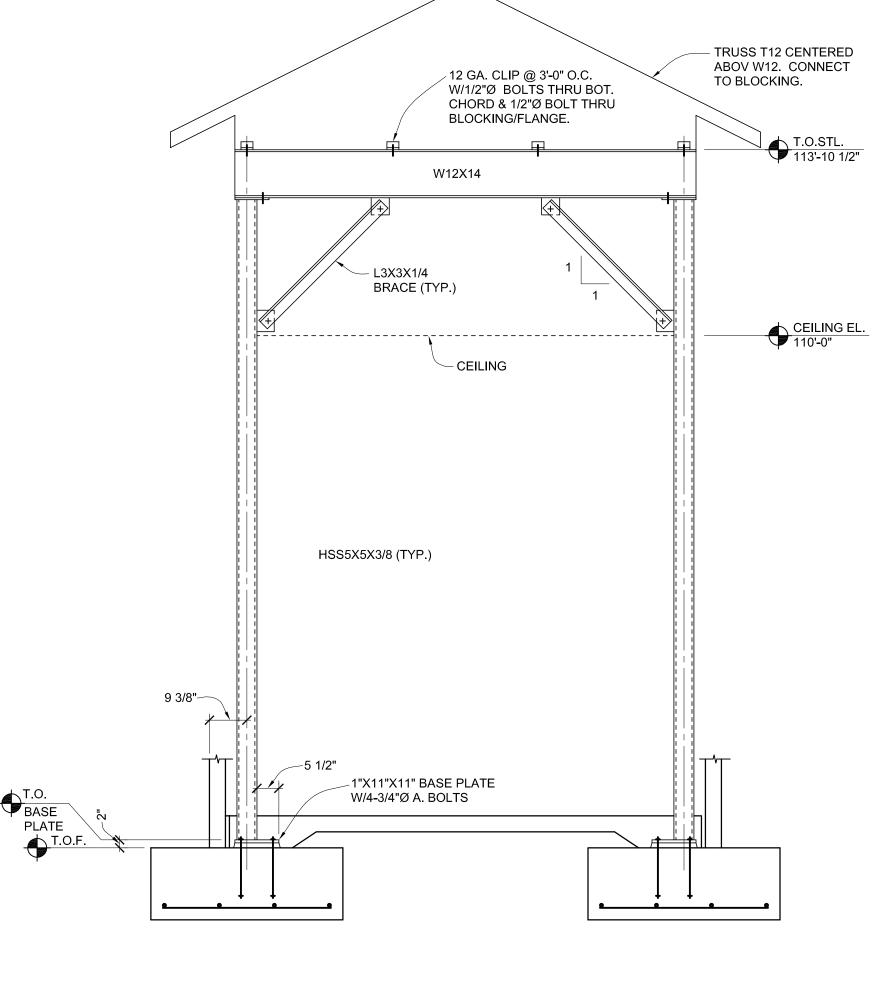
WELDED WIRE FABRIC.

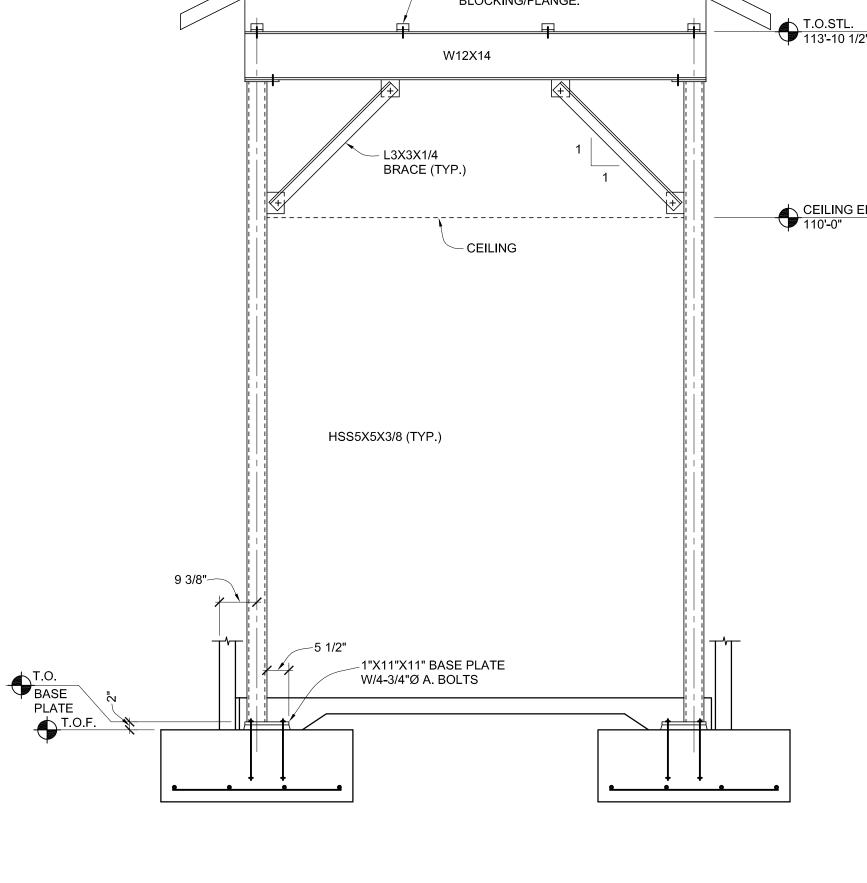
- CAST-IN-PLACE CONCRETE - HOLLOW CMU - 100% SOLID OR GROUT FILLED CMU

- EXISTING WALL

HSS6X6X5/16 (CURVED) T.O. OPENING SEE ARCH. DRAWINGS L6X6X5/16 (CURVED) -FULL HEIGHT MC6X12 JAMB. CONNECT TO FOUNDATION W/L3X3X1/4 CLIP & 1/2"Ø EXP. BOLT.

SECTION  $1 \ 1/2" = 1'-0"$ 







KORDA/NEMETH **ENGINEERING. INC.** 1650 WATERMARK DRIVE COLUMBUS, OHIO 43215 Jeffrey A. Kaeppner JOB FILE

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KORDA/NEMETH

Checked KORDA/NEMETH

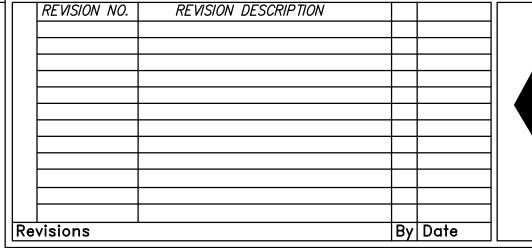
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Department of Veterans Affairs **Medical Center** 1826 Veterans Blvd. **Dublin, Ga. 31021** 



	1. MEDICAL
	2. ASSOCIA
NG, LLG	3. CHIEF C
REET	4. ASSOC.
•	5 SERVICE

Recommended Approvals:		Drawing Title
1. MEDICAL DIRECTOR	6. OPERATIONS SERVICE LINE	STŘUCTURAL NOTES
2. ASSOCIATE DIRECTOR	7. INFECTION CONTROL	
3. CHIEF OF STAFF	8. SAFETY MANAGER	Submittal:  CONSTRUCTION BID DOCUMENTS
4. ASSOC. DIRECTOR	9. GENERAL ENGINEER	Job Number
5. SERVICE LINE MGRS.	10. COTR	2009-05-01



